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Loss factor prediction for laminated plates

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Analysis and modelling

ABSTRACT

Purpose: The study aims to predict the loss factor properties of composite laminated plates.

Design/methodology/approach: Elastic constants of laminates and damping properties have been determined by using an identification procedure based on multi-level theoretical approach.

Findings: The present paper is the first attempt at proposing a novel adaptive procedure to derive loss factor parameters for sandwich plate's vibration.

Research limitations/implications: In the future the extension of the present approach to sandwich plates with different core materials will be performed in order to test various sandwich design.

Practical implications: Structures composed of laminated materials are among the most important structures used in modern engineering and especially in the aerospace industry. Such lightweight and highly reinforced structures are also being increasingly used in civil, mechanical and transportation engineering applications.

Originality/value: The main advantage of the present method is that it does not rely on strong assumptions on the model of the plate. The key feature is that the raw models can be applied at different vibration conditions of the plate by a suitable analytical or approximation method.

Keywords: Computational material science; Composite materials; Laminated plates; Elastic constants

<u>1.Introduction</u>

The simple laminate theories [1-7] are most often incapable of determining the 3-D stress field in the lamina. Thus, the analysis of composite laminates may require the use of laminate independent theory or a 3-D elasticity theory. Exact three-dimensional solutions [8-18] have shown the fundamental role played by the continuity conditions for the displacements and the transverse stress components at the interfaces between two adjacent layers for an accurate analysis of multilayered composite thick plates. Further, these elasticity solutions demonstrated that the transverse normal stress plays a predominant role in these analyses. However, accurate solutions based on the three-dimensional elasticity theory are often intractable.

The limitations of the analysis based on the displacement formulation motivated the work [19-20] in which the mixed variation theorem for the dynamic analysis of multilayered plates was used. A semi-analytical method has been developed in to obtain the natural frequencies of vibration of simply- supported laminated composite cross-ply plates. Continuity of the transverse stresses as well as the displacements have been explicitly satisfied at the lamina interfaces in these models. Further, these models have been formulated by considering a local Cartesian co-ordinate system at the mid-surface of each individual layer. Six degrees of freedom (DOF), viz. three displacement components u, v and w (along x, y and z directions, respectively) and three transverse stress components are expressed at the bottom as well as the top surface of each individual layer. The time dependent axial and the transverse displacements along the x, y and z directions of any point can be expressed using power series expansions [8-14].

2.Some aspects of the laminated beam modeling

Exact static solutions for composite laminates in cylindrical bending. Bending by moment. Let us consider the exact solution for uniform cylindrical bending of layered symmetric composite laminates. The governing equations are:

$$\frac{\partial \sigma_{xx}}{\partial X} + \frac{\partial \tau_{xz}}{\partial Z} = 0, \quad \frac{\partial \tau_{xz}}{\partial X} + \frac{\partial \sigma_{zz}}{\partial Z} = 0.$$
(1)

$$\sigma_{xx} = C_{xx}\varepsilon_{xx} + C_{xz}\varepsilon_{xz}, \ \sigma_{zz} = C_{xz}\varepsilon_{xx} + C_{zz}\varepsilon_{zz}, \tau_{xz} = G\gamma_{xz} \quad .$$
(2)

If the stress-strain state is uniform, then these relations must hold, because τ on the face plate surfaces is equal to 0

$$\frac{\partial \tau}{\partial Z} = -\frac{\partial \sigma_{xx}}{\partial X} = 0 \quad and \quad \tau = 0,$$
(3)

$$\frac{\partial \sigma_{zz}}{\partial Z} = -\frac{\partial \tau}{\partial X} = 0 \quad and \quad \sigma_{ZZ} = 0 \tag{4}$$

If the stress σ_{xx} is not equal to 0, the following assumption must be made

$$\sigma_{xx} = S(z). \tag{5}$$

From Eq.(1-5) we may obtain for the displacement

$$U = XZ, W = -0.5 \cdot \left(\alpha Z^2 + X^2\right), \quad \alpha = C_{xz} / C_{zz}$$

$$\sigma_{xx} = Z (C_{xx} - \alpha C_{xz})$$
(6)

For the bending moment we obtain

$$M = \int_{-H}^{H} Z^2 \cdot (C_{xx} - \alpha C_{xz}) dZ$$
⁽⁷⁾

Here for the material non homogeneity $C_{xx}(Z)$, $\alpha(Z)$, $C_{xz}(Z)$ all may be functions of the normal coordinate Z of the plate. Thus, by comparison with a uniform Tymoshenko beam of the same depth, an equation for the bending rigidity of a uniform equivalent beam may be written

$$EI_{T} = \frac{M}{d\gamma} = \int_{-H}^{H} Z^{2} \cdot (C_{XX} - \alpha C_{XZ}) dZ , \qquad (8)$$

because $\frac{d\gamma}{dx} = 1$.

Bending by force. In this case the primary assumptions are

$$\sigma_{xx} = x \cdot S(z), \qquad \tau = T(z) \tag{9}$$

From Hooks law we obtain

$$\varepsilon_{x} = \frac{\partial u}{\partial x} = xS^{*}, \ \varepsilon_{z} = \frac{\partial w}{\partial z} = -\alpha_{1}xS^{*};$$

$$S^{*} = \frac{S(z)}{1 - \alpha_{1}\alpha_{2}}, \quad \alpha_{1} = \frac{C_{xz}(z)}{C_{xx}(z)}, \quad \alpha_{2} = \frac{C_{xz}(z)}{C_{zz}(z)} \quad .$$
(10)

Next by integration for the displacement we obtain

$$u = \frac{x^2}{2}S^* + \varphi(z), \quad w = -x \int_0^z \alpha_1 S^* dz + \psi(x) \quad . \tag{11}$$

By substituting Eq.(11) into Eq.(1) we can derive (for a symmetrically laminated plate, but the same may be made for arbitrary lamination)

$$u = \frac{Ax^2 z}{2C_x} + A_0^{\tilde{z}} \left[\int_0^{z} \frac{\alpha_2 z}{C_x} dz + \frac{\tau}{G} \right] dz - \Phi z ,$$

$$w = -\frac{Ax^3}{6C_x} - Ax \int_0^{z} \frac{\alpha_2 z}{C_x} dz + \Phi x$$
(12)

where for the tangential stress au and the constant Φ we have

$$\tau = -A \int_{z}^{H} (1 - \alpha_{1}\alpha_{2}) z dz , \Phi = \frac{A}{H} \int_{0}^{H} dz \left(\int_{0}^{z} \frac{\alpha_{2}z}{C_{x}} dz + \frac{\tau}{G} \right) dz$$
(13)

In the above equations A is an arbitrary constant number. If we equate the tangential force Q to I, then from (12,13)

$$Q = \int_{-H}^{H} \tau dz = 1 \implies A = \left(\int_{-H}^{H} \left(\int_{z}^{H} (1 - \alpha_{1} \alpha_{2}) z dz \right) dz \right)^{-1}$$
(14)

Thus, by comparison with a uniform Tymoshenko beam of the same cross section, we may write an equation for the transverse rigidity of a uniform equivalent beam

$$\frac{1}{2HG_T} = -A_0^{\tilde{z}} \frac{\alpha_2 z}{C_x} dz + \Phi =$$

$$= -A_0^{\tilde{z}} \frac{\alpha_2 z}{C_x} dz + \frac{A}{H} \int_0^H dz \left(\int_0^{\tilde{z}} \frac{\alpha_2 z}{C_x} dz + \frac{\tau}{G} \right) dz$$
(15)

where τ is given by Eq.(13). It may be seen that from Eq.(12) we may obtain the same bending rigidity as in Eq.(8).

Asymptotic approach. Above (Part 1) we have obtained the exact solution for the statically loaded sandwich plate by the cylindrical bending. The equivalent Tymoshenko beam is established with the same bending and tangential rigidity. Naturally, by the dynamic loading the static approach is incorrect. It may be valid only for the thin plates and lower frequency excitation. As was said previously, many authors have maid such a work. But there assumption were submerged or to one layer model, or to hypothesis like [1-7]. Such theories based on mixed approach or another variation principles [19] are not simply proved mathematically. Thus, here we should applied Galerkin method to linear elasticity dynamic problem [20]. By the Banach fixed-point theorem [20] a) problem has a unique solution; b) we have a priory estimate; c) convergence of the Galerkin method. Let us consider now such kinematical assumptions (for symmetrical three layered plate)

$$U_{e} - \begin{cases} u = \sum_{i,k} u_{ik}^{e} z^{i} \sin k\pi x / L, & 0 < z < H, \\ w = \sum_{i,k} w_{ik}^{e} z^{i} \cos k\pi x / L, & 0 < x < L, \end{cases}$$
(16)

$$U_{d} - \begin{cases} u = \sum_{i,k} u_{ik}^{d} z^{i} \sin k\pi x / L, \\ 0 < z < H_{p}, \\ w = \sum_{i,k} w_{ik}^{d} z^{i} \cos k\pi x / L \quad 0 < x < L. \end{cases}$$
(17)

By substituting Eq.(1,2) to

$$\int_{V} (\sigma_{xx} \delta \xi_{xx} + \sigma_{zz} \delta \xi_{zz} + \tau_{xz} \delta \xi_{xz} - \rho \frac{\partial u}{\partial t} \delta \frac{\partial u}{\partial t} - \rho \frac{\partial w}{\partial t} \delta \frac{\partial w}{\partial t}) dV = 0, \quad (18)$$

and by assuming the unifrequency vibration we obtain the set of linear algebraic equations

$$[A]u = \begin{bmatrix} A_1 & A_d \\ A_d^T & A_2 \end{bmatrix} u^e = 0$$
(19)

For grater number of lamina may be wrote

$$U_{d}^{n} - \begin{cases} u = \sum_{i,k} u_{ik}^{n} z_{n}^{i} x^{(2k-1)}, & 0 < z < H_{p}, \\ w = \sum_{i,k} w_{ik}^{n} z_{n}^{i} x^{(2k-2)}, & 0 < x < L, \\ w = \sum_{i,k} w_{ik}^{n} z_{n}^{i} x^{(2k-2)}, & n = 1, ..., N - 1, \end{cases}$$
(20)

where

$$H_p^n = H - \sum_{l=1}^{n-1} h_l \ . \tag{31}$$

Now

The frequency equation should be written such

$$\left[\left[K \right] - \omega^2 \left[M \right] \right] \left\langle q_0 \right\rangle = \left\langle 0 \right\rangle \tag{22}$$

Damping. Loss factors is found by analytical solutions (1-15)

$$\eta_{\Sigma} = \frac{1}{\eta_{1}} \frac{\frac{L^{3}}{3} \int_{0}^{H} \eta \sigma_{XX} \varepsilon_{XX} dz + L \int_{0}^{H} \eta \tau^{2} / G dz}{L^{3}} \frac{L^{3}}{3} \int_{0}^{H} \sigma_{XX} \varepsilon_{XX} dz + L \int_{0}^{H} \tau^{2} / G dz}.$$
(23)

In Fig.1. the loss factor η_{Σ} (Eq.16) is presented for the beams with the various mechanical properties of cover layers. Nondimension mechanical parameters were: Cxx(1)=1, Cxx(2)=10. Another parameters (*Czz*, *Cxz*, *G*) were changed.

In Fig.2 the loss factors are shown for the three-layered beam with thin elastic interlayer (fife-layered beam). The depth of the interlayer was 0.03H. Mechanical properties of the interlayers were: $Cxx_i/Cxx(2) = G_i/G(2) = Cxz_i/Cxz(2) = Czz_i/Czz(2) = 0.1$.



Fig.1. (a) loss factor in a symmetrical three-layered beam (EL/H=1/30); (b) loss factor in a symmetrical three-layered beam (EL/H=1/3)



Fig.2. (a) loss factor in a symmetrical fife-layered beam (changing properties of the face; (b) loss factor in a symmetrical fife-layered beam (changing properties of the face layers,EL/H=1/30)

3.Conclusions

Various displacement models have been developed by considering combinations of displacement fields for in-plane and transverse displacements inside a layered beam to investigate the phenomenon of vibrations in laminated composite plates. Numerical evaluations are obtained for loss factor of three- an fife-layer plates.

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